#### Case Study: Long-term Performance of SMA Pavements in Washington State

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### Outline

- Introduction
- Project Information
- Research Scope
- Results of SMA and HMA Comparison
  - Field Performance
  - Field Cores Mixture Properties
  - Extracted Binder Properties
- Conclusions and Future Study

# Introduction

- SMA is widely used in northern and central Europe for over 25 years.
- In U.S., some studies in MD and GA showed: SMA performs well against rutting and roughness for periods exceeding 10 years.
  - ✓ Stone to stone contact
  - ✓ High asphalt content; Polymer modified binder
- National specifications: AASHTO R46, AASHTO M325
- State's good experience is critical for successful implementation of SMA.
- WA's experience (not so good at the beginning):
  ✓ 1999: SR 524 mix design construction issue
  - ✓ 2000: I-90 inadequate control over mix production

# **Project Information**

- Eastern Washington: dry-freeze
- I-90: from SR 21 to Ritzville; AADT- 38,300; paved in 2001
- SMA: 12.5-mm NMAS, PG 76-28
- HMA: 12.5-mm NMAS, PG 64-28
- Both on WB lanes, overlay thickness 63.5 mm





# **Research Objective**

 Investigate the long-term performance of SMA pavement as compared to control HMA pavement



#### WSPMS

- Pavement structural condition (PSC): cracking
- Pavement rutting condition (PRC): rutting
- Pavement profile condition (PCC): roughness
- Field inspection



- Field cores
  - Mixtures testing
  - Binder extraction
  - Aggregate gradation
  - Binder Recovery
    - Binder testing

#### **Material Characterization: Mixture**

Mixture Test	IDT Dynamic Modulus/Creep Compliance	Fatigue- IDT Fracture at Room Temp	Thermal Cracking- IDT Fracture at Low Temp	Studded Tire Wear Test
Testing Conditions	Temp.: -20, -10, 0, 10, 20, 30°C; Frequency: 20, 10, 5, 1, 0.1, 0.01 Hz Duration: 100 seconds	Temp.: 20ºC Loading rate: 50.8 mm/min	Temp.: -10ºC Loading rate: 2.54 mm/min	Temp.: room Pressure: 690 kPa Speed: 140 rpm Duration: 2 min
Material Properties	Dynamic modulus; Creep compliance	IDT strength; Fracture work density; Horizontal failure strain	IDT strength; Fracture work density	Mass loss
References /Standards	Wen et al. 2002	Shen et al. 2017; AASHTO T322	Shen et al. 2017; AASHTO T322	Wen and Wu 2015





**Vertical Failure Deformation** 



#### **Material Characterization: Asphalt Binder**

Binder Test	Performance Grading (PG)	Rutting: MSCR	Fatigue: Monotonic at Room Temp	Thermal Cracking: Monotonic at Low Temp	
Testing Conditions	Different temp depending on the test (DSR, BBR)	Stress: 0.1, 3.2 kPa Temp.:	Temp.: 20ºC Shear rate: 0.3 s <sup>-1</sup>	Temp.: 5°C Shear strain rate: 0.01 s <sup>-1</sup>	
Material Properties	PG; BBR stiffness; m-value	Jnr <sub>0.1</sub> , Jnr <sub>3.2</sub> ; R <sub>0.1</sub> , R <sub>3.2</sub>	Maximum stress; Fracture energy; Failure strain	Maximum stress; Fracture energy; Failure strain	
References/Stan dards	AASHTO MP1/T240/T313	AASHTO T350	Shen et al. 2017	Wu 2017; Shen et al. 2017	





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#### **Field Performance**



### **Dynamic Modulus**



- Overall, HMA E\* 20% higher than SMA E\*.
- SMA is more flexible than HMA.

# **Creep Compliance**



- Overall, HMA shows lower creep compliance than SMA.
- SMA gives a better ability to relax stress, and thus better thermal cracking resistance.

#### **Studded Tire Wear Test Result**



- No significant difference in mass loss
- Comparable wear resistance

	Average Mass	Average Mass Standard		
	Loss, g Deviation, g		F-value	
11 HMA specimens	2.7	1.46	$0.72 \times a = 0.05$	
12 SMA specimens	3.3	0.75	$0.73 > \alpha = 0.05$	

# **IDT Test Results**

20°C





Test	Daramotors	H№	1A	SN	SMA	
Condition	Farameters	Mean	σ	Mean	σ	SMA, %
20°C	IDT Strength, kPa	2992.3	297.2	2581.4	74.5	15.9
	Fracture Work Density, kPa	148.9	24.8	220.6	2.8	-32.5
	Horizontal Failure Strain	0.0060	0.0004	0.0096	0.0014	-37.5
-10° C	IDT Strength, kPa	4465.0	369.6	4397.5	188.2	1.5
	Fracture Work Density, kPa	82.0	11.0	120.0	9.0	-31.6

 SMA performs better than HMA for bottom-up and top-down cracking resistance, as well as thermal cracking resistance.

### **Aggregate Gradation Test Result**





	In-place Measured	Designed Asphalt
	Asphalt Content, %	Content, %
SMA	6.8	6.8
HMA	5.6	5.44

#### **Binder PG Test Results**

	Original PG	Measured True PG	PG
HMA	64-28	73.3-24.4	70-22
SMA	76-28	81.8-29.3	76-28

• SMA slows down oxidation possibly due to a thicker asphalt film.

#### **Binder MSCR Test Results**



• SMA binder shows better resistance to rutting.

#### **DSR Monotonic Fracture Test Result**

1600 004 004 004 004 004 004 004	20°0		HMA SMA	4000 3500 3000 2500 1500 1000 500	5°(	C	HMA SMA
0 5 10 15 20 25 30 35 40 Shear Strain			<sup>0</sup> Shear Strain <sup>10</sup> <sup>15</sup>				
Binder	SMA	HMA	SMA – HMA, %	Binder	SMA	HMA	SMA – HMA, %
Shear Strength, kPa	1446	1256	15	Shear Strength, kPa	2410	4144	-42
Fracture Energy, kPa	10495	1930	444	Fracture Energy, kPa	5275	5082	4
Failure Strain	10	2	443	Failure Strain	3	1	85

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## Conclusions

- SMA pavement exhibited better long-term field performance than HMA control pavement.
- Field SMA field cores indicated:
  - ✓ Lower E\* and higher creep compliance
  - ✓ Better resistance to rutting
  - ✓ Comparable resistance to studded tire wear
  - ✓ Better resistance to bottom-up and top-down fatigue cracking
  - ✓ Better thermal cracking resistance
- Field-extracted SMA binder indicated:
  - ✓ Slower oxidation rate due to a thicker film thickness
  - ✓ Better rutting resistance
  - ✓ Better fatigue and thermal cracking resistance

#### Future Study: Balanced Mix Concept for SMA



(Credit: Mr. David Lippert)

#### **Balanced Mix Design Concept for SMA**





# **Future Study**

- Include more case studies with varying traffic, environmental and other factors to draw relatively conclusive decisions.
- Further evaluation on the effects of aggregate gradation and binder PG on the difference performance.

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# Thank You! Any questions?



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